

Model Answer: Summer 2018

Subject: Basic Surveying

Sub. Code: 22205

Important Instructions to examiners:

- 1) The answers should be examined by key words and not as word-to-word as given in the model answer scheme.
- 2) The model answer and the answer written by candidate may vary but the examiner may try to assess the understanding level of the candidate.
- 3) The language errors such as grammatical, spelling errors should not be given more importance. (Not applicable for subject English and Communication Skills.)
- 4) While assessing figures, examiner may give credit for principal components indicated in the figure. The figures drawn by the candidate and those in the model answer may vary. The examiner may give credit for any equivalent figure drawn.
- 5) Credits may be given step wise for numerical problems. In some cases, the assumed constant values may vary and there may be some difference in the candidate's answers and the model answer.
- 6) In case of some questions credit may be given by judgment on part of examiner of relevant answer based on candidate's understanding.
- 7) For programming language papers, credit may be given to any other program based on equivalent concept.

Que. No.	Sub. Que.	Model Answers	Marks	Total Marks
Q.1		Attempt any five of the following:		10
	(a) Ans.	Define "Chain survey". Chain Survey: The Survey in which only linear measurements are taken in the field with the help of chain and remaining work such as plotting, calculation etc. are done in the office, is called as chain survey.	2	2
	(b) Ans.	Enlist various methods of levelling.Methods of leveling:i. Simple levellingii. Differential levellingiii. Longitudinal levellingiv. Cross section levellingv. Fly levellingvi. Check levellingvii. Reciprocal levellingviii. Profile levelling	½ each (any four)	2
	(c) Ans.	 Define plane survey and geodetic survey. Plane survey: The survey in which curvature of the earth is not taken in to consideration is called as plane survey. Geodetic survey: The survey in which curvature of the earth is taken in to consideration is called as geodetic survey. 	1	2
	(d) Ans.	State any two causes of local attraction. Local attraction caused due to attraction of magnetic needle by following sources:		



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Que. No.	Sub. Que.	Model Answers	Marks	Total Marks
Q. 1		 i. Iron or steel structure ii. Electric cable line iii. Wires iv. Iron poles v. iron ore vi. Bunch of key vii. Knife viii. Steel Tape ix. Earth's gravitational force of attraction 	1 each (any two)	2
	(e) Ans.	 State meaning of i) A scale of plan ii) Drawing to scale. i. A Scale of plan: A scale of plan is a ratio of linear dimensions of an object as represented in drawing to actual dimensions of the same object. ii. Drawing to scale: The proportion by which the drawing of an object is enlarged or reduced is called as drawing to scale. 	1	2
	(f) Ans.	Define "Contour" and "Contour line". Contour: An imaginary line on the ground, joining the points of same elevation or same R.L's is called as 'Contour'.	1	
		Contour line: A line passing through points of equal elevation or equal R.L's is called as contour line. $\frac{OR}{N}$ The line of intersection of a level surface with ground surface is known as contour line.	1 1	2
	(g) Ans.	 Enlist the components of digital planimeter. Components of digital planimeter: Digital display Rolling wheel or Rollers Tracing arm Functional keys or buttons Sliding wheel 	¹ /2 each (any four)	2
Q. 2	(a) Ans.	Attempt any three: Define: i) Magnetic bearing ii) FB iii) BB iv) Bearing of line.		12
		i. Magnetic bearing; The bearing of a line measured with respect to magnetic north in clockwise manner is called as magnetic bearing.	1	
		ii. Fore bearing: The bearing observed in the direction of progress of survey with respect to north direction in clockwise manner is called fore bearing.	1	



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110.	Que.	iii. Back bearing : The bearing observed in the backward direction or opposite direction of survey with respect to north direction in clockwise manner is called back bearing.	1	4
		iv. Bearing of line: The horizontal angle made by survey line with respect to meridian (North direction) is known as bearing of line.	1	
	(b) Ans.	Explain the principle of surveying. Principle of surveying are as follows: To work from whole to part. According to the first principle, the whole area is first enclosed by main stations and main survey lines as shown in figure above. The area is then divided into a number of parts by forming well conditioned triangles. A nearly equilateral triangle is considered to be the best well conditioned triangle. The main survey lines are measured very accurately with a standard chain. The sides of triangles are measured. The purpose of this process of working is to prevent	2	
		 accumulation of error. ii. To locate a new station by at least two measurement from fixed reference points. accumulation of error. accumulation of error. better the station of the error o	2	4



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Sub. Que.	Model Answers	Marks	Total Marks
(c) Ans.	 Explain temporary adjustment of dumpy level. Temporary adjustment of dumpy level: Setting up the level. The level fixed on tripod. The legs of tripod stand are well spread so that the level will remain stable on tripod. Bring all the three foot screws in the Centre of their run so that they can be turned clockwise or anticlockwise as required, for Levelling purpose. Adjust the height of the instrument so that the observer can Comfortably see through the telescope and note the readings. Fix two legs of tripod and adjust third leg in such a way that the levelling head will become as horizontal as possible by eye judgment. 	1	
	 II. Levelling up the level. i. The base of the tripod is already leveled with the help of cross bubble. ii To make accurate adjustment of the level, the longitudinal level is adjusted in the Centre of its run, with the help of three foot screws. iii. Make the bubble parallel to the any selected pair of foot screws. Now; turn both the foot screws either inward or outward with the help of foot screws till the bubble appears in the center. iv. Turn the telescope through 90° and now with the help of third screw bring the bubble of levelling tube in the center. v. Repeat above process, until bubble will remain at centre in both position. Then levelling is said to be completed. 	1	4
	III. Focusing the eye piece.i. Hold a sheet of white paper in front of the objective glass 4 to 6 cm away from objective glass and see through the eye piece.ii. Turn the eye piece inwards or outwards in the socket so that the cross hair on the diaphragm appears sharp and clear.	1	
	 IV. Focusing the object glass. i. Direct the telescope towards any object, say a levelling staff in the field which is kept at a distance. See through eyepiece whether the staff is visible, distinct or not. ii. If not, then turn the focusing screw till the image is distinct and clear. The cross hair on the diaphragm should also be seen clearly. 	1	
	Que. (c)	Que. Model Answers (c) Explain temporary adjustment of dumpy level. Ans. I. Setting up the level. i. The level fixed on tripod. ii. The legs of tripod stand are well spread so that the level will remain stable on tripod. iii. Bring all the three foot screws in the Centre of their run so that they can be turned clockwise or anticlockwise as required, for Levelling purpose. iv. Adjust the height of the instrument so that the observer can Comfortably see through the telescope and note the readings. v. Fix two legs of tripod and adjust third leg in such a way that the levelling head will become as horizontal as possible by eye judgment. II. Levelling up the level. i. The base of the tripod is already leveled with the help of cross bubble. iii To make accurate adjustment of the level, the longitudinal level is adjusted in the Centre of its run, with the help of foot screws. Now; turn both the foot screws either inward or outward with the help of foot screws. iii. Make the bubble parallel to the any selected pair of foot screws. Now; turn both the foot screws either inward or outward with the help of third screw bring the bubble of levelling tube in the center. iv. Turn the telescope through 90° and now with the help of third screw bring the bubble of levelling tube in the center. v. Repeat above process, until bubble will remain at centre in both position. Then levelling is said to be completed. I	Que. Model Answers Marks (c) Explain temporary adjustment of dumpy level. Temporary adjustment of dumpy level: I. Setting up the level. i. The level fixed on tripod. ii. The legs of tripod stand are well spread so that the level will remain stable on tripod. iii. Bring all the three foot screws in the Centre of their run so that they can be turned clockwise or anticlockwise as required, for Levelling purpose. 1 iv. Adjust the height of the instrument so that the observer can Comfortably see through the telescope and note the readings. 1 v. Fix two legs of tripod and adjust third leg in such a way that the levelling head will become as horizontal as possible by eye judgment. 1 II. Levelling up the level. i. The base of the tripod is already leveled with the help of cross bubble. 1 ii To make accurate adjustment of the level, the longitudinal level is adjusted in the Centre of its run, with the help of foot screws. Now; turn both the foot screws either inward or outward with the help of foot screws till the bubble appears in the center. 1 iv. Turn the telescope through 90° and now with the help of third screw bring the bubble of levelling tube in the center. 1 iv. Turn the telescope through 90° and now with the help of third screw bring the bubble of levelling tube in the center. 1 iv. Turn the telescope through 90° and now with the clep of third screw bring the bubble of levelling tub



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Que. No.	Sub. Que.			Model Answers		Marks	Total Marks
Q. 2	(d) Ans.			g bearings from WCB to QB 57°45'iii) 51°15'iv) 333°30'			
		Sr. No.	WCB	Conversion	QB		
		i.	$\Theta_1 = 247^{\circ}30^{\circ}$	As Θ_1 lies in III rd Quadrant RB = Θ_1 - 180° = 247°30° - 180° = S 67°30° W	S 67°30' W	1	
		ii.	Θ ₂ = 167°45'	As Θ_2 lies in II nd Quadrant RB = 180° - Θ_2 = 180° - 167°45' = S 12°15' E	S 12°15' E	1	4
		iii.	=N 51°15'E	N 51°15'E	1		
		iv.	Θ ₄ = 333°30'	As Θ_4 lies in IV th Quadrant RB = 360°- Θ_4 = 360° - 333°30° = N 26°30° W	N 26°30' W	1	
Q. 3	(a) Ans.	Explai i. ii. iii.	As bench mark known or preas taken on it. Th height of instr useful to calcul levelling. Thus bench m work. It is not ground points v The BM is imp arithmetical ch fall method.	of benchmark in levelling. is the ground point whose ele- ssumed; the back sight reading is back sight (BS) reading is u ument (HI) in line of collima- late R.L.'s of other ground poin ark (BM) is essential to comr possible to calculate reduced without knowing BM. portant to check the RL calcula ecks in both methods i.e. HI r eful to check the one day's obs	g (first reading) is iseful to calculate ation. This HI is ints, in all types of mence the survey levels (RL's) of tions by applying method and Rise-	each	12



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Que. No.	Sub. Que.	Model Answers	Marks	Total Marks
Q. 3	(d)	Draw sketch of dumpy level and name all parts.		
	Ans.	 i clescope <li clescope<="" li=""> i clescope i clescope <li< th=""><th>4</th><th>4</th></li<>	4	4
Q. 4		Attempt any three:		12
	(a)	Differentiate between height of instrument and rise and fall		
	Ans.	method.		
		Height of Instrument Method Rise and Fall Method		
		IThis method is a fast method and is less tedious because it requires less calculations.IThis method is a slower method than H.I. method as it involves more calculations.IIThere is no check on n.L.s of intermediate stationsIIThere is a complete check on all calculation work.		
		IIIFollowing applied, $\Sigma BS - \Sigma FS$ IIIFollowing applied, $\Sigma BS - \Sigma FS$ = Last R.L First R.L.= $\Sigma Rise - \Sigma Fall$ = Last R.L First R.L.	1 each (any four)	4
		IVError in calculations of RLs of intermediate stations is not carried forward.IVError in calculations of RLs of intermediate stations is carried forward.VThismethodisnon-		
		V This method is less accurate. V This method is more accurate. VI It is used for calculations of profile levelling in construction works such VI It is used for calculations of precise levelling works, check		



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Q. 4	(b)	Explain procedure for computing volume by i) Trapezoidal formula ii) Prismoidal formula		
	Ans.	I. Trapezoidal formula: Procedure for computing the volume by trapezoidal formula is as follows: i. Divide the total length of plan (L-section) in to number of strips (cross sections). In this method areas are divided into series of trapezoids as shown in figure below. ii. Calculate the areas of each section (i.e. ends , intermediate from A ₁ to A _n) iii. Calculate volume of earth work in cutting and embankment by using formula. Trapezoidal formula: $V = D/2 \times (A_0 + 2A_1 + 2A_2 + \dots + 2A_{n-1} + A_n)$	2	
		Where, $A_{0,,} A_{1,}A_{n-1} A_n$ –Areas of cross sections. $A_{0,,} A_{1,}A_{n-1} A_n$ –Areas of cross sections. In this method areas are divided into series of prismoids as shown in		4
		 II. In this include areas are divided into series of prismolas as shown in figure below. II. Prismoidal formula: Procedure for computing the volume by prismoidal formula is as follows: Divide the total length of plan (L-section) in to number of strips (cross sections). In this method areas are divided into series of prismoids as shown in figure above. Calculate the areas of each section (i.e. ends, intermediate from A₁ to A_n) Calculate volume of earth work in cutting and embankment by 	2	
		using formula. iv. Prismoidal formula : $V = D/3 \times (A_0 + 4(A_1 + A_3 +) + 2(A_2 + A_4 +) + A_n)$		



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Que. No.	Sub. Que.	Model Answers	Marks	Total Marks
		Model Answers Explain four uses of contour map. Following are uses of contour map: i To draw longitudinal section and plan of given map. ii To determine inter-visibility between two points. iii To trace contour gradient and to locate route for alignments of railways, roadways, canals etc. iv To measurement of drainage areas. v. To calculate reservoir capacity. vi To find intersection of surfaces and measurement of earth work. vii To determine nature of ground in proposed area.	1 each (any four)	



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Que.				Model	Answers			Marks	Total Marks
(d)	staff of 0.76, 1 0.875, RL of 1	n contin 1.515, 1.9 1.085, 1.' first poir	uously s 935, 2.40 790, 2.45 nt is 200.	lopping)0, 2.985 0. 500 m.	ground at 5, 3.650, 1.	common in	evel on 4 m terval 30 m. 2.495, 3.57,		
Ans.	Staff Stn.	BS	IS	FS	HI	RL	Remark		
		0 760			201.26	200 500	First RL		
	-	0.700	1 515		201.20		THERE		
		1 015	2.000	3 650	198 625		CP1	3	
		1.010	1 855	5.000	190.020				
		0.875	2.190	3 570	195 930		CP2		
		0.070	1.085	0.070	1,00,00				
									4
				2.450			Last RL		
		2.650							
(e) Ans.	2.65 – 9 - 7.02 = Descril The pro as follo i. Take that wh ii. Start will be iii. Set iv. Ma point o v. Press end it a	9.67 = 19 = - 7.02 be proce ocedure o ows: the area nile measure the plar displaye the scale rk one s f magnifi s the star	dure for of measu on the p urement i imeter b d. by press tarting po er of trac button a	measuri rement o plane sur it does no y pressin ing scale oint on t cing arm and move g point. P	ng area usi f an area u face of tabl of move. g on button button on k boundary of of digital pl tracing arm ress the end	sing digital e and fix it on key pad ey pad. f that area a animeter. on boundar	planimeter is with clips so of it. Screen and place the ry of area and	1	4
		0.875, RL of Calcul Ans. Staff Stn. 0 30 60 90 120 150 180 210 240 270 300 300 300 210 240 270 300 300 300 210 240 270 300 300 300 300 300 300 300 210 240 270 300 300 300 300 300 300 2.65 - - 7.02 = Markethat whii. Star will be ii. iii. Set iv. Ma <t< td=""><td>(e) 0.875, 1.085, 1.' RL of first point Ans. Staff Stn. BS 0 0.760 30 60 90 120 150 1.015 180 210 240 0.875 270 300 300 330 Σ 2.650 Check: Σ BS - Σ FS = I 2.65 - 9.67 = 19 -7.02 = -7.02 Describe proce The procedure of as follows: i. Take the area that while measure ii. Start the plant will be displayed iii. Set the scale iv. Mark one step ont of magnified</td><td>(e) 0.875, 1.085, 1.790, 2.45 RL of first point is 200. Calculate RL of all point of magnifier of trade (e) Staff BS 1.855 210 2.400 1.20 2.985 150 1.015 1.855 210 2.495 240 0.875 270 1.085 300 1.790 300 1.015 1.805 210 2.495 240 0.875 270 1.085 300 1.790 330 2.400 1.855 210 2.495 240 0.875 2.400 1.085 300 1.790 330 S 2.650 S 1.085 300 1.790 330 S 2.650 Check: S S S S S S 1.085 1.085 1.025 1.0</td><td>(e) 0.875, 1.085, 1.790, 2.450. RL of first point is 200.500 m. Calculate RL of all points by HI Ans. $\$ \frac{Staff}{Stn.}$ BS IS FS 0 0.760 - - - 30 1.515 - - - 60 1.935 - - - - 90 2.400 - - - - - 120 2.985 -</td><td>(e) 0.875, 1.085, 1.790, 2.450. RL of first point is 200.500 m. Calculate RL of all points by HI method. Ans. \overline{Staff} BS IS FS HI 0 0.760 201.26 30 1.515 0 60 1.935 0 201.26 30 1.515 0 0 120 2.985 0 0 120 2.985 0 0 150 1.015 3.650 198.625 180 1.855 0 0 240 0.875 3.570 195.930 270 1.085 0 0 300 1.790 0 0 300 2.450 Σ 2.650 9.670 Check: Σ 2.650 9.670 Σ (e) Ans. Describe procedure for measuring area usi The procedure of measurement of an area u as follows: i. Take the area on the plane surface of table that while measurement it does not move. ii. Start the planimeter by pressing on button will be displayed. iii. Set the scale by pressing scale button on k iv. Mark one starting point on boundary of point of magnifier of traci</td><td>0.875, 1.085, 1.790, 2.450. RL of first point is 200.500 m. Calculate RL of all points by HI method. Ans. Staff BS IS FS HI RL 0 0.760 201.26 200.500 30 1.515 199.745 60 1.935 199.325 90 2.400 198.860 120 2.985 198.275 150 1.015 3.650 198.625 180 1.855 196.770 210 2.495 196.130 240 0.875 3.570 195.930 195.055 270 1.085 194.845 300 1.790 194.845 300 1.790 194.845 300 1.790 194.845 300 1.790 194.845 196.700 193.480 \$\scilpsilpsilpsilpsilpsilpsilpsilpsilpsilps</td><td>RL of first point is 200.500 m. Calculate RL of all points by HI method. Staff BS IS FS HI RL Remark 0 0.760 201.26 200.500 First RL 30 30 1.515 199.745 60 1.935 199.325 90 2.400 198.860 120 2.985 199.325 90 2.400 198.625 197.610 CP1 180 1.855 196.770 10 210 2.495 196.130 240 0.875 3.570 195.930 195.055 CP2 270 1.085 194.845 300 1.790 194.140 330 2.450 193.480 Last RL 2 2.650 9.670 193.480 Last RL 2.655 -9.67 = 193.48 - 200.50 -7.02 = -7.02 Check: Cscribe procedure for measuring area using digital planimeter. The procedure of measurement of an area using digital planimeter. The procedure of measurement of an area using digital planimeter is as follows:</td><td>(e) (f) (f) (f) (f) (f) (f) (f) (f</td></t<>	(e) 0.875, 1.085, 1.' RL of first point Ans. Staff Stn. BS 0 0.760 30 60 90 120 150 1.015 180 210 240 0.875 270 300 300 330 Σ 2.650 Check: Σ BS - Σ FS = I 2.65 - 9.67 = 19 -7.02 = -7.02 Describe proce The procedure of as follows: i. Take the area that while measure ii. Start the plant will be displayed iii. Set the scale iv. Mark one step ont of magnified	(e) 0.875, 1.085, 1.790, 2.45 RL of first point is 200. Calculate RL of all point of magnifier of trade (e) Staff BS 1.855 210 2.400 1.20 2.985 150 1.015 1.855 210 2.495 240 0.875 270 1.085 300 1.790 300 1.015 1.805 210 2.495 240 0.875 270 1.085 300 1.790 330 2.400 1.855 210 2.495 240 0.875 2.400 1.085 300 1.790 330 S 2.650 S 1.085 300 1.790 330 S 2.650 Check: S S S S S S 1.085 1.085 1.025 1.0	(e) 0.875, 1.085, 1.790, 2.450. RL of first point is 200.500 m. Calculate RL of all points by HI Ans. $$ \frac{Staff}{Stn.}$ BS IS FS 0 0.760 - - - 30 1.515 - - - 60 1.935 - - - - 90 2.400 - - - - - 120 2.985 -	(e) 0.875, 1.085, 1.790, 2.450. RL of first point is 200.500 m. Calculate RL of all points by HI method. Ans. \overline{Staff} BS IS FS HI 0 0.760 201.26 30 1.515 0 60 1.935 0 201.26 30 1.515 0 0 120 2.985 0 0 120 2.985 0 0 150 1.015 3.650 198.625 180 1.855 0 0 240 0.875 3.570 195.930 270 1.085 0 0 300 1.790 0 0 300 2.450 Σ 2.650 9.670 Check: Σ 2.650 9.670 Σ (e) Ans. Describe procedure for measuring area usi The procedure of measurement of an area u as follows: i. Take the area on the plane surface of table that while measurement it does not move. ii. Start the planimeter by pressing on button will be displayed. iii. Set the scale by pressing scale button on k iv. Mark one starting point on boundary of point of magnifier of traci	0.875, 1.085, 1.790, 2.450. RL of first point is 200.500 m. Calculate RL of all points by HI method. Ans. Staff BS IS FS HI RL 0 0.760 201.26 200.500 30 1.515 199.745 60 1.935 199.325 90 2.400 198.860 120 2.985 198.275 150 1.015 3.650 198.625 180 1.855 196.770 210 2.495 196.130 240 0.875 3.570 195.930 195.055 270 1.085 194.845 300 1.790 194.845 300 1.790 194.845 300 1.790 194.845 300 1.790 194.845 196.700 193.480 \$\scilpsilpsilpsilpsilpsilpsilpsilpsilpsilps	RL of first point is 200.500 m. Calculate RL of all points by HI method. Staff BS IS FS HI RL Remark 0 0.760 201.26 200.500 First RL 30 30 1.515 199.745 60 1.935 199.325 90 2.400 198.860 120 2.985 199.325 90 2.400 198.625 197.610 CP1 180 1.855 196.770 10 210 2.495 196.130 240 0.875 3.570 195.930 195.055 CP2 270 1.085 194.845 300 1.790 194.140 330 2.450 193.480 Last RL 2 2.650 9.670 193.480 Last RL 2.655 -9.67 = 193.48 - 200.50 -7.02 = -7.02 Check: Cscribe procedure for measuring area using digital planimeter. The procedure of measurement of an area using digital planimeter. 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Q. 5		Attempt any two			12	
	(a)	Calculate includ	traverse and apply usual			
		Line	FB	BB		
		AB	46 ⁰ 30'	226 ⁰ 30'		
		BC	117 ⁰ 30'	298 ⁰		
		CD	168 ⁰	349 ⁰		
		DA	290 ⁰	112 ⁰ 30'		
	Ans.	$\angle A = BB \text{ of } DA$	$- FB of AB = 112^{0}30'$ -	$46^{0}30' = 66^{0}$		
		$\angle B = BB \text{ of } AB$	$-FB \text{ of } BC = 226^{\circ}30'$ -	$117^{0}30' = 109^{0}$		
		$\angle C = BB \text{ of } BC$	2			
		$\angle D = BB \text{ of } CD$				
		Check:				
		Sum of included				
		$\angle \mathbf{A} + \angle \mathbf{B} + \angle \mathbf{B}$	$\mathbf{C} + \mathbf{\Delta} \mathbf{D} = (\mathbf{2n} - 4) \mathbf{x}$	00 ⁰	2	6
		$66^0 + 109^0 + 130^0$	$-59^0 = (2 \times 4 - 4) \times 90^0$			U
		$364^0 = 360^0$				
		Total error = 364	$4^{\circ}-360^{\circ}=4^{\circ}$			
		Correction for ea	ach angle = $4^0/4 = 1^0$			
		Hence correction	of -1 ⁰ should be applied			
		Corrected $\angle A =$	$66^{\circ} - 1^{\circ} = 65^{\circ}$			
		Corrected $\angle B =$	$109^{\circ} - 1^{\circ} = 108^{\circ}$		2	
		Corrected $\angle C =$	$130^{\circ} - 1^{\circ} = 129^{\circ}$			
		Corrected $\angle D =$	$59^0 - 1^0 = 58^0$			
		Sum of included	angle = $(2n - 4) \times 90^{\circ}$			
			z C + z D = (2n - 4) x	90 ⁰		
			$-58^0 = (2 \times 4 - 4) \times 90^0$			
		$360^0 = 360^0$	× /			



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Que. No.	Sub. Que.				lodel Aı				Marks	Total Marks
Q. 5	(b)		ll reading B	are in 'm'. 17	iff surve	C57		ilate area in		
		< A		25 45 F 24		67 с	78 E 15	100 D		
	Ans.	А	0	B 17 b 25	45 f	57 67 c	78	e 100 D	1	
		Sr.			24 F		E	15		6
		No.	Figure	Chainage	Base	Offsets	Offset	Area		
		1	Δ AbB	0-25	25	0 & 17	8.5	212.5		
		2	\Box bBCc	25-67	42	17 & 57	37	1554	5	
		3	$\frac{\Delta CcD}{\Delta DeE}$	67-100 78 - 100	33 22	57 & 0 15 & 0	28.5 7.5	940.5 165	0	
		4		45-78	33	24 & 15	19.5	643.5		
		6	ΔAfF	0-45	45	0 & 24	12.	540		
			- / 111	0 10	10		al Area	4055.5 m ²		
	(c)	contin 0.950 0.925 The I book	nuous slop , 1.615, 1.9 , 1.455, 1.7 RL of firs and enter and fall m	ing staff on 2.450, 3.750, evel of field all points by ng first and						



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Que. No.	Sub. Que.				Mo	del Ans	wers				Marks	Total Marks
Q. 5	Ans.	Sr. No.	Chainage	BS	IS	FS	Rise	Fall	RL	Remark		
		1	0	0.950					100.000	First RL	_	
		2	25		1.615			0.665	99.335			
		3	50		1.925			0.310	99.025			
		4	75		2.515			0.590	98.435			
		5	100		2.895			0.380	98.055		_	
		6	125	1.125	1 0 0 0	3.495		0.600	97.455	C.P.1	_	
		7	150		1.980			0.855	96.600		4	
		8 9	175 200	0.925	2.450	3.750		0.470	96.130 94.830	C.P.2	_	
		9 10	200	0.925	1.455	3.730		0.530	94.830	C.P.2	_	
		10	223		1.435			0.330	94.005		_	
		12	230		1.750	2.850		1.100	92.905	Last RL	_	6
		12	Σ	3.000		10.095	0	7.095	72.705	Eust RE		0
							Ť				-	
			dient = (Las = (92 = -0.0 = -1 /) – 10.0 ⁴ st RL – 1 .905 – 1 258 38.75	95 = 0 -7.095 = First RI 00.000	- 7.095 = -7.095 L) / Dista	= 92. = -7.0	905 – 1		R.L.	1	



Subject: Basic Surveying

Que. No.	Sub. Que.	Model Answers	Marks	Total Marks
Q. 6	(a)	Attempt any two: Counter survey data of a field is shown in given figure. Draw		12
		94.000 m contour line by linear interpolation method. Show all the calculations grid size is 10 m x 10 m. 90.00 A B 95.00		
		91.275 C D 96.135		
		94. 030 E F 97.815		
	Ans.	95.00 95.00 94.00 94.00 10	1½	
		96.135 94.00 96.135 - 91.275 $\frac{96.135 - 91.275}{10} = \frac{94.00 - 91.275}{x}$ $\frac{96.135 - 91.275}{10} = \frac{2.725}{x}$ x = 5.606 m	1½	6
		94.030 94.00 94.00 94.00 94.00 - 91.275 $\frac{94.030 - 91.275}{10} = \frac{94.00 - 91.275}{x}$ $\frac{2.755}{10} = \frac{2.725}{x}$ x = 9.891 m	1½	



Subject: Basic Surveying

Que. No.	Sub. Que.	М	Marks	Total Marks		
Q. 6		90.00 A 8.				
		91.275 C 5.6	06 m	D 96.135	11/2	
		9.891 m 94.00 94. 030 E		F 97.815		
	(b)	Following bearing were				
		ABCDE. Calculate included				
			e bearing	Back bearing		
			$10^{0} 00'$	290 ⁰ 00'		
			0 ⁰ 15' 44 ⁰ 00'	$\frac{214^{0} 15}{64^{0} 00}$		
			14 00 10 ⁰ 15'	$\frac{64\ 00^{\circ}}{130^{\circ}\ 15'}$		
			92 [°] 45'	14 ⁰ 45'		
	Ans.	$\angle A = FB \text{ of } AB - BB \text{ of } EA$				
	1 111.5.	$\angle B = 360^{\circ} - (FB \text{ of } BC -$				
		100 ⁰ 15'				
		$\angle C = FB \text{ of } CD - BB \text{ of } BC$	2			
		$\angle D = (FB \text{ of } DE - BB \text{ of } C$				
		$\angle E = FB \text{ of } EA - BB \text{ of } DE$				
		Check:				<i>,</i>
		Sum of included angle = (2n		6		
		$\angle A + \angle B + \angle C + \angle D =$				
		95°45' +100°15'+29°45' +11				
		$534^{0}30' = 540^{0}$				
		Total error = $534^{0}30^{\circ}-540^{0}$ =				
		Correction for each angle =	2			
		Hence correction of 1 ⁰ 6' shou	uld be applie	d		
		Corrected $\angle A = 95^{\circ}45'+1^{\circ}6'$	$= 96^{0}51$			
		Corrected $\angle B = 100^{\circ}15'+1^{\circ}$				



Subject: Basic Surveying

Que. No.	Sub. Que.	Model Answers									Total Marks
Q. 6		Correct									
		Correct	ted $\angle D$	$= 246^{\circ}15$	$+1^{0}6'=2$.47 ⁰ 21'					
		Correct	ted $\angle E$	$= 62^{\circ}30^{\circ}$	$+1^{0}6' = 63$	3°36'					
										2	
		Sum of									
		$\angle A +$									
		$96^{0}51' + 101^{0}21' + 30^{0}51' + 247^{0}21' + 63^{0}36' = (2 \times 5 - 4) \times 90^{0}$ $540^{0} = 540^{0}$									
	(c)	Find t check.	he usual								
		Stn.	B.S.	I.S.	F.S.	Rise	Fall	R.L.	Remark	-	
		1	2.345					129.50	BM1		
		$\begin{vmatrix} 2\\ 3 \end{vmatrix}$	1.650	2 210	X	0.035	X			-	
		4	X	2.210	1.850	X	Λ			-	
		5	1.850		1.925		0.455		C.P.		
		6			Х	0.37		129.00			
	Ans.	2.345 - X = 0.035 X = 2.310 1.650 - 2.210 = X X = 0.560 2.210 - 1.850 = X X = 0.360 X - 1.925 = -0.455 X = 1.470								21/2	
		1.850 -	-X = 0.37 X = 1.4								



Subject: Basic Surveying

Que. No. Q. 6	Sub. Que.	Model Answers								Marks	Total Marks
		Stn.	B.S.	I.S.	F.S.	Rise	Fall	R.L.	Remark		
		1	2.345					129.500	First RL		
		2	1.650		2.310	0.035		129.535	CP1		
		3		2.210			0.560	128.975		11/	(
		4	1.470		1.850	0.360		129.335	CP2	11/2	6
		5	1.850		1.925		0.455	128.880	CP3		
		6			1.480	0.370		129.250	Last RL		
		Σ	7.315		7.565	0.765	1.015				
		C			65=0.76	Σ Rise - Σ F 55 - 1.015 5 = -0.25 =	= 129.50	R.L. – First – 129.25	R.L.	2	