

# SUMMER-2014 EXAMINATIONTotal Pages-10Subject Code : 17420

### Model Answer : Geotechncial Engineering

Important instruction to examiners:

- The answers should be examined by key words and not as word to word as given in the model answer scheme.
- The model answer and the answer written by candidate may vary but the examiner may try to assess the understanding level of the candidate.
- The language errors such as grammatical, spelling errors should not be given more importance (Not applicable for subject English and Communication Skills).
- 4) While assessing figures, examiner may give credit for principal components indicated in the figure. The figures drawn by the candidate and model answer may vary. The examiner may give credit for any equivalent figure drawn.
- Credits may be given stepwise for numerical problems. In some cases the assumed constant values may be vary and there may be some difference in the candidates answer and model answer.
- 6) In case of some questions credit may be given by judgment on part of examiner of relevant answer based on candidates understanding.
- For programming language papers, credit may be given to any other program based on equivalent concept.



Q.NO	Code: 17420Model Answer (Geotechnical Engineering)PageSOLUTION	MARK
a)	Attempt any six of the following	12 M
i)	Define geology Geology : is the science that deals with the study of the earth as a planet. Thus, it includes essence of scientific studies dealing with the origin, age and structure of the earth.	2 M
ii)	State classification of rocks based on their genesis : i) Igneous rock ii) Sedimentary rock iii) Metamorphic rock	2 M
iii)	<b>Define Faults</b> : The fractures along which there has been relative movements of the blocks past each other are termed as faults.	2 M
iv)	<ul> <li>State any two points stating importance of structural geology.</li> <li>i) Geology provides a systematic knowledge of construction materials, their structure and properties.</li> <li>ii) The knowledge of erosion, transportation and deposition of surface water helps in soil conservation, river control, coastal and harbor works.</li> <li>iii) The knowledge about the nature of the rocks is very necessary in tunneling , constructing roads and in determining the stability of cuts and slopes.</li> <li>iv) The foundation problems of dams, bridges and buildings are directly related with geology of the area where they are to be built.</li> </ul>	2 M (any two)
v)	Enlist two methods used to determine bulk density and dry density. i) Core cutter method ii) Sand replacement method	1 M 1 M
vi)	<b>State IS code definition of soil</b> Soil is defined as the sediment or other unconsolidated accumulation of solid particles produced by physical and chemical disintegration of rock.	2 M
vii)	State any four field applications of geotechnical engineering. i) In foundation design ii) In pavement design iii) In earth retaining structures iv) In design of earthen dams i) In design of embankments ii) In design of under ground structures	Any four points (1/2 each) 2 M
	<b>Define water content.</b> It is defined as the ratio of the weight of water to the weight of solids in a given soil mass W = (Ww/Ws) * 100	2 M



1 b		Attempt	t any two of the following	the fight stream and	are and the second second	2/10			
	i)		various types of rocks with respect to th	eir engineering u	ses				
		Types of rocks							
		i) Igneous rock : ii) Sedimentary rock : iii) Metamorphic rocks :							
		, 0	us rock :						
		Many of igneous rocks, where available in abundance are extensively used as material for construction. Granitas, sympletes and delerites are characterized by your high							
		for construction. Granites, syenites and dolerites are characterized by very high crushing strength and hence can be easily trusted in most of construction works. Basalts and other dark coloured igneous rocks, through equally strong not be used in residential building but find much use as foundation and road stones							
			mentary rock :	ad stones					
			tary rocks are formed by accumulation,	compaction and	consolidation of	1M			
			ts. Sedimentary rocks possesses good wa						
		which ro	cks can be used in reservoir or dam con	struction.					
			morphic rocks :						
			extremely fine grained metamorphic ro						
			tion purpose. Quartzite are granular met			1 M			
		construct	ense and uniformity grained. It finds extense	ensive use in buil	ding and road				
	ii)		four types of folds and explain any one			1.4			
	11)		etrical folds ii) Asymmetrical folds iii) Over	turned folds		1 (types) 3 (explain			
			nal folds v) Recumbent folds vi) Conjugat			any one)			
	iii)	Explain s	salient features of earthen dam in Mahar	ashtra (any two d	ams)	Any			
	10 20	Sr. No.	Features	Panset	Chaskaman	four			
		1	Length of dam	1039 m	1045 m	points			
		2	Volume content of Dam $(10^3 \text{ m}^3)$	4190	2903	(1/2each for each			
		3	Gross Storage capacity $(10^3 \text{ m}^3)$	303000	318.17	dam			
		4	Reservoir area $(10^3 \text{ m}^3)$	15645	18218	uam			
		5	Effcetive storage capacity $(10^3 \text{ m}^3)$	294000	210.99	4 M			
		0	Purpose	Irrigation and	Irrigation and				
		7	Designed spillway capacity (m <sup>3</sup> /sec)	water supply 1162	power generation 3962				
			Designed spinway capacity (in /sec)	1102	3902				
<b>).</b> 2		Attempt	any four of the following	ierral storid be m		16			
				ed spicester	0.25504.20030.15	Marks			
	a)	•	soil in details :						
			e grained soils : i) Gravel ii) Sand						
		-	aded clean (W)			$1\frac{1}{2}M$			
	28.80	-	aded with excellent clay binder (C) graded, fairly clean (P)						
			vered in other groups						
			<b>rained soils :</b> i) Inorganic silt an very fi	ine sands (M)		1 ½ M			
		~) e	ii) Inorganic clay ( C) iii)		clay (O)	1 /2 111			
		c) Highly	y organic soils and other miscellaneou			1 M			
		,		which the soft est	te leaned solare   10)				
M	b)	Explain	two causes and two effects of earthqu	ake					
MT	b)	Causes o	f earthquake: i) Movement of tectonic	plates ii) Volcan	1 /	2 (Explain			
MT	b)	Causes o Anthropo		plates ii) Volcan	1 /	2 (Explain any two )			
M	b)	Causes o Anthropo fluids	f earthquake: i) Movement of tectonic	plates ii) Volcan sives vi) Injectior	and Extraction of				



	(ISO/IEC - 27001 - 2005 Certified) 3	110
c	Classify seismic waves in Indian earthquake	1
	<b>P</b> - Waves : These are the primary waves by which material particle undergo extensional and compressional strains along the direction of the energy transmission due to which the particles vibrates in the longitudinal direction	1 M
	<ul> <li>S - Waves : These are the secondary waves by which material particles oscillate at right angle to the direction of the energy transmission due to which the shearing of particles occurs in the transmission direction.</li> <li>L - Waves : These are the surface waves, called</li> </ul>	1 M
	as Love waves, by which the displacement of material particles is practically horizontal due to which shearing rupture takes place. <b>Rayleigh Waves :</b> These are the surface waves by which the material particles oscillates in an elliptic path in the vertical plane. These are travels faster than L –Waves.	1 M
d)	Explain what do you mean by earthquake resistance structures	1 M
	<ul> <li>Earthquake damages the structures like dam, buildings, kills people. The conventional buildings instantly collapse without any warning, during the unpredictable and sudden earthquake.</li> <li>For the safe earthquake resistance design, follow design principles and zoning as per the provision made in various codes published by Bureau of Indian Standards. The Indian seismic codes are IS : 1893, IS:4326, IS : 13827, IS : 13828, IS : 13920 and IS : 13935.</li> <li>Some of the following points are to be followed to construct the safe earthquake resistance structures.</li> <li>i) The foundation should be provided over the hard rock with no sign of faults.</li> <li>ii) The building should be symmetrical and rectangular in plan</li> <li>iii) Minimum 16 mm diameter reinforcing bar should be used in all structural member of RCC building.</li> </ul>	2 M 2 M
	iv) Ductile material should be provided to construct the structures	10.2
e)	<b>Define Focus and epicenter</b> <b>Focus :</b> The focus of an earthquake, also called its "hypocenter", is the point below ground level where the earthquake originates and generates shock waves which generates in all direction, e.g. where the snag on the fault shears.	2 M
f)	<b>Epicenter:</b> The epicenter is the point at the ground level, immediately above the focus. <b>Define – Atterberg's limit of consistency</b>	2 M
1)	Atterbergs limits : The water content at which the soil changes from one state to another are known as Consistency limits or Atterbergs limits. Liquid Limit : The water content at which the soil changes from the liquid state to plastic state is known as liquid limit (LL, w <sub>L</sub> ). In other wards the liquid limit is the	1M 1M
	water content at which the soil ceases to be liquid. <b>Plastic Limit :</b> The water content at which the soil becomes semisolid is known as the plastic limit. (PL,w <sub>p</sub> ). The plastic limit is the water content at which the soil just fails to behave plastically. Soil begins to crumble when rolled into a thread of 3 mm diameter. The numerical difference between the liquid limit and the plastic limit is known as plasticity index. (PI, Ip), $PI = LL - PL$ . <b>Shrinkage limit:</b> The water content at which the soil changes from a semisolid state to	1M



	the solid state is known as the shrinkage limit (SL, w <sub>s</sub> ).	4/10
	Shrinkage limit is the smallest water content at which a reduction in water content will not cause a decrease in the volume of the soil mass. At this water content the shrinkage ceases.	1M
2.3	Attempt any four of the following	16 M
a)	<ul> <li>analysis as per IS code method.</li> <li>Procedure of Mechanical Sieve analysis for fine grained soil</li> <li>Materials and Equipment:</li> <li>Balance accurate to 1 gm, set of sieves 4.75 mm, 2.36 mm, 1.18mm, 600 micron, 300 micron, 150 micron, 75micron, receiver, metal trays, mechanical sieve shaker etc.</li> <li>Procedure:</li> <li>Take a representative sample of soil received from the field and dry it in the oven. Break the clods of the sample by means of hand.</li> <li>Weigh the required amount of sample for testing say 5 kg.</li> <li>The sample is sieved through the set of sieves arranged in descending order of their sieves.</li> <li>The portion retained on 4.75 mm and retained on 75 micron sieve is sand fraction.</li> <li>These fractions are expressed by weight of original sample to give gravel content and sand content in percentage.</li> <li>The weight of the soil portion retained on each sieve and pan is obtained to the nearest 0.1 gm.</li> </ul>	1 M 3 M
	- The weight of the retained soil is checked against the original weight. <b>Note:</b> If the soil contains appreciable fine (75%) aggregates and hard to break in to elementary particles soak the sample for 24 hours and wash through 75 microns sieve. The residue on the sieve is weighed.	
b)	Explain Darcy's law of permeability It states that for laminar flow, the rate of flow or discharge per unit time is directly proportional to the hydraulic gradient. Mathematically, q = Kia	
	where, q = discharge per unit time. K = Darcy's coefficient of permeability, i = Hydraulic gradient a = Total cross section of soil mass perpendicular to the direction of flow.	2 M
c)	Explain various factors affecting permeabilityi) Grain sizeii) Properties of pore fluidiii) Temperatureiv) Void ratiov) Stratification of soilvi) Entrapped air and organic impuritiesvii) Adsorbed waterviii) Degree of saturationix) Shape of particlesx)Structures of soil mass	4 (explain any 4, 1 man each )
d)	Define phreatic line and flow line. Draw neat sketch of flow net.         Phreatic Line : When the earthen structures are constructed to retain water, the topmost flow line through the soil mass is called as phreatic line.         Flow net : The grid mesh or net formed by the intersection of equipotential lines and flow lines is called as flow net.         Field         Field         Field         Field	1 M (Def.) 1 M (Def.) 2 Fig



soil.	5/10
i) Cohesive soil :	
$\tau_{\rm f} = C + \sigma \tan \phi$	
where,	
$\tau_{\rm f} = { m Shear strength}$	11
$\sigma = Nornal stress$	
$\phi$ = Angle of shearing resistance	
C = Cohesion	
$T_{1} = C + \sigma \tan \phi$	
t = C + O	11
σ	
cohesion soil	
ii) Cohesive less soil.	
$\tau_{\rm f} = \sigma \tan \phi$	
where,	
$\tau_{\rm f}$ = Shear strength	1N
$\sigma = Nornal stress$	
$\phi$ = Angle of shearing resistance	
$\tau_{\bigstar}$	
T = otano	
	1M
$\overline{\sigma}$	
Cohesionless soil	
) Define earth pressure. Enlist and define its types.	
Earth pressure : Retaining structure does resist the lateral pressure exerted by the soil	
mass. This lateral pressure is known as earth pressure.	1M
Types :	
i) Lateral earth pressure : Soil in contact with any vertical or inclined face of	1M
structure exert force on structure which is known as lateral earth pressure	A TAR
<b>II) Active earth pressure :</b> As pressure exerted on retaining wall resulting from slight	1 M
movement of wall away from filling	T 141
iii) Passive earth pressure : When movement of the retaining wall is such that the soil	1 M
tends to compress horizontally.	T TAT



		(ISO/IEC	- 27001 - 2005 Certified)	6	110
Q .	SOLU	TION	STOR DETERMENT		MARKS
Q.4	Effect	of water table on bearing capacity of soil		C. 2012/05/07	
. a)	1.	The rise in water table from below the f	foundation results in decrease in bearing	, canacity	mã (m
Sol		in granular soil.	is an accrease in bearing	; capacity	Any
=>	2.	When the water table reaches the groun	d where the denth is greater of equal to	width of	Four
		footing the bearing capacity is reduced b	v 50% or more	width of	1M
	3.	The bearing capacity is not affected for p	ourely cohesive soil		each
	4.	The bearing capacity for non-granular so	il decreases with presence of water tabl		cacin
	5.	Presence of water table for shallow de	nth give noor bearing capacity as com	norad for	adde (adda
		larger depth foundation.	put give poor bearing capacity as com	pared 101	Territor 1
Q.4	Assum	aptions in Terzaghis analysis	and the second secon		
. b)		The soil is homogeneous and isotropic a	and its shear strength is represents by C	aulomb'a	A
Sol		equation.	and its shear strength is represents by C	oulomb's	Any
=>	2	The strip footing has rough base and the	problem in accontially two dimensional		Four
	3.	The shear strength of soil above the base	of footing is perforted. The seil share	.4.1	1M
	5.	is replaced by a uniformity surcharge $\gamma$ E	e of footing is neglected. The soll above	the base	each
	4	The load on the footing is vertical and is			orienees (
		The footing is long i.e. L/B ratio is infin	nite where <b>D</b> is the width and <b>L</b> is the	1. 1. 0	
	5.	footing.	inte, where B is the width and L is the	length of	
	6	The elastic zone has straight boundarie	as inclined at $w = w$ to the heritagetal		
	0.	plastic zones fully developed.	is meanied at $\psi = \psi$ to the norizontal,	, and the	in the second second
Q.4	Sr.No		Canadidation		
. c)	1		Consolidation		And and a
Sol	1	Instant compression of soil under	Gradual compression under steady		
=>		dynamic load is called as	load is called as consolidation.		
	2	compactionThe settlement of structure of			
	2		The settlement of structure takes		Any
	3	prevented due to compaction.	place due to consolidation.		Four
	3	Compaction is carried out for	Consolidation occurs naturally due		1Mark
		improving properties of soil.	to structural load and it does not		each
	4		improve properties of soil.		
	4	Compaction is an artificial process.	Consolidation is a natural process.		
	5	Compaction is done before	Consolidation takes place after		
		constructing any structure.	constructing the structure.		
		action Curve : -	20 80% 100% Degree of sat	uration	
Q.4		he soil is 100% saturated then the dry			Any
d)		y of soil is calculated for 100%	19 - onlinums XXXX		Two
Sol		tion and resulting line on compaction			1Mark
-	curve	is called as Zero air void line.	20 18 - Z	RO AIR	each
			(B) of a V	DIDLINE	
		e zero air void line indicates that the	(B) What is the second		Dig
		dry density cannot reach theoretical	test		2Marks
	value	even after heavy compaction.	16 (A)	4	
			Standard proctor test		
		ending upon the compactive efforts on	15 <u>10 5 10 15 20</u>	<u>ì.</u>	
		he graph can be plotted for light	. 0 5 10 15 20 Water content, w. %	25	
	compa	ction and for heavy compaction			
					1995 ·



### MAHARASHTRA STATE BOARD OF TECHNICAL EDUCATION (Autonomous)

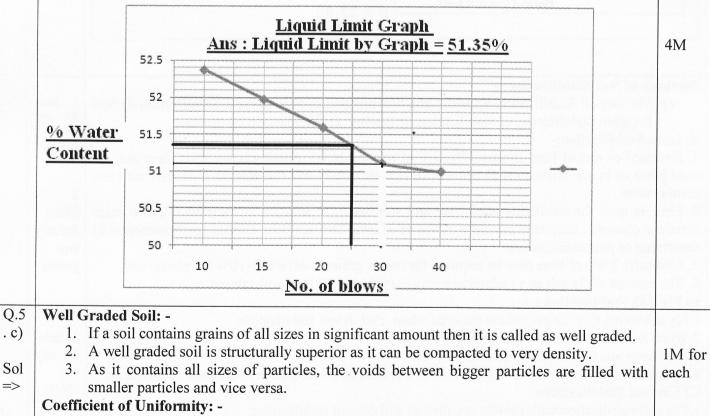
(ISO/IEC - 27001 - 2005 Certified) 7/10 Q.4 Types of Compaction Equipment: -. e) i) Compaction by Rolling: -Sol a) Smooth wheel rollers : => Suitability: These rollers best suitable for 5ubgrade or base coarse compaction of cohesion less soils., Any 2 b) Pneumatic tyred rollers: 2Mark Suitability: Pneumatic tyred rollers are effective for compacting cohesive as well as each cohesion less soils. Light rollers are effective for compacting soil layers of small thickness c) Sheep foot roller : Suitability : Suitable only for fine grained soil ii) Compaction by Rammers: Rammers or tampers are mainly two types, hand operated and mechanical rammer. A hand operated rammer consists of a block of iron or stone about 3 to 5 kg in mass, attached to a wooden rod. The tamper is lifted for about 0.3 m and dropped on the soil to be compacted. A mechanical rammer is operated by compressed air or gasoline power. It is much heavier, about 30 to 50 kg. Ramming equipment's consists of three types: dropping weight type, internal combustion type and pneumatic type. Rammers or tampers are used to compact the soil. Suitability: Suitable for all types of soil iii) Compaction by vibratory compactors : The vibrating equipment, mounted on screeds, plates or rollers are of two Types: a) Dropping weight type and b) Pulsating hydraulic type. By giving vibration to Soil, soil particles are packed together and compaction of soil is achieved. Suitability: Suitable for compacting granular soils. with no fines in layer up to 1 m thickness **O.4** Hand Operated Augers: -. f) Sol => Extension Helical auger uge 2 mark Post hole auge Each Fig 1: Hand Augers Given : -Q.5 V=100cc, G=2.68, W=190gm,  $W_s=160gm$ . e=?, W=?S=?. a) Sol 2 M  $W = \frac{W_w}{W_s} = \frac{30}{160} = 18.75 \%$ => 1 M  $\gamma = \frac{W}{V} = \frac{190}{100} = 1.90 \text{ gm/cc}$ 1 M  $\gamma_d = \frac{W_s}{V} = \frac{160}{100} = 1.6 \text{ gm/cc}$  $\gamma_d = \frac{G \gamma_w}{(1+e)}$ ,  $e = \frac{G \gamma_w}{\gamma_d} - 1$  $e = \frac{2.68 \, x \, 1}{1.6} - 1 = 0.675$ 2 M  $S = \frac{W \times G}{e} = \frac{0.1875 \times 2.68}{0.675} = 0.744 \%$ 2M



Q.5 . b) Sol =>

## **Observation Table :**

Sr. No	Determination Number	1	2	3	4	5	
1	Number of blows	40	30	20	15	10	4 M
2	Container No.	1	2	3	4	5	
3	Mass of container (g) W3	5	5	5	5	5	
4	Mass of container + Wet soil (g) W1	30.67	32.2	31.3	32.75	30.05	
5	Mass of container + Dry soil (g) W2	22	23	22.35	23.26	21.44	
6	Mass of water (g) W1 -W2	8.67	9.2	8.95	9.49	8.61	
7	Mass of oven dry soil (g) W2 - W3	17	18	17.35	18.26	16.44	
8	W1 -W2	51.00	51.11	51.59	51.97	52.37	
	Water content % x 100 W2 - W3	-					



- 1. It is defined as the ratio of  $D_{60}$  to  $D_{10}$  size of given soil.
- 2. The coefficient of uniformity is nearly unity.

3. 
$$C_u = \frac{D_{60}}{D_{10}}$$

=>

8/10

1M each



### MAHARASHTRA STATE BOARD OF TECHNICAL EDUCATION (Autonomous)

	(Autonomous) (ISO/IEC - 27001 - 2005 Certified)	110
	Where $D_{10} = 10\%$ of particles are finer than that size. $D_{60}$ = means that soil for which the total smaller particles in given soil are 60%	
	Coefficient of Curvature: - 1. It represents shape of particle size distribution curve. 2. $C_c = 1$ to 3 for well graded soil. $C_c > 4$ for well graded gravel. $C_c >$ for well graded sand. $C_c = \frac{D_{30}^2}{D_{30}^2}$	2 M
Q.6	$C_c = \frac{D_{30}^2}{D_{10} \ X \ D_{60}}$	
(2.0 .a) Sol =>	i)Angle of Shearing Resistance: $-\emptyset = 22^{\circ}$ from graph $m = \frac{y_2 - y_1}{x_2 - x_1} = \frac{6.8 - 3.6}{10 - 2}$ $\emptyset = tan^{-1}(m)$ $\emptyset = tan^{-1}(0.4)$ $\emptyset = 21.80$ ii)Cohesion : - $S = C + \sigma \tan \emptyset$ $3.6 = C + 2 \ge 0.4$ Cohesion $C = 2.8$ N/mm <sup>2</sup> From Graph : $\emptyset = 21.80$ C = 2.8	4 M for graph 4M for analyti cal calcula tion
Q.6 . b) Sol =>	1. Hydrated or slaked lime is very effective in treating heavy plastic clayey soil. Lime may be used alone or in combination with cement, fly-ash etc. sandy soil can also be stabilized with this	2 Mark for any four types
	combination. 2. Lime is used for stabilizing road base and sub base. On addition of lime to soil two main reactions occurs a) alteration in nature of absorbed layer through Base Exchange phenomenon b) cementing or puzzolanic action.	2 Mark for any two points
	<ul> <li>1.Fly ash forms filler or pozzolana material when used in soil stabilization.</li> <li>2.When it is used in soil stabilization it hydrates.</li> <li>3.It is cheap and eco-friendly material.</li> <li>4.Fly ash is rarely used in soil stabilization as it is used as an admixture to enhance stabilization.</li> <li>C) Cement Stabilization: -</li> <li>1.The soil stabilization with cement is called as soil cement stabilization.</li> <li>2.When Cement stabilization is used there is large increase in strength.</li> </ul>	2Marks for any two points
	3.Cement stabilization is used for loose soil and non-cohesive soil. 4.Cement stabilization is not useful for clay, organic soil and expansive soil.	2 Marks for any



10	110
Note:- For Explaining any other method use same marking scheme	two
	points
Assumptions of Rankines Theory: -	Any
	four
2. The soil element is in the state of plastic equilibrium.	4M
3. The ground surface is plane which may be horizontal.	
4. The back of is vertical and smooth.	
5. The wall yield about the base thus satisfies deformation condition for plastic equilibrium.	
Rankines theory for cohesion less backfill: -	
a) Dry Backfill	
b) Submerged Backfill	
c) Surcharge for horizontal Plane surface.	
b) Submerged backfill : -	2M for
1. In submerged backfill the land behind the wall is saturated with water and lateral pressure	two
is having pressure due to submerged unit weight and water.	points
2. If water is present on both side of wall then the pressure is not to be considered.	each
c) Surcharge for horizontal plane pressure: -	2M for
1. The back fill is horizontal and carries a surcharge of uniform intensity as per unit area.	two
2. The pressure at the base of retaining wall consists of pressure made of unit weight of	points
backfill and pressure due to uniform surcharge.	
Note:- For Explaining any other method use same marking scheme	
	<ul> <li>Note:- For Explaining any other method use same marking scheme</li> <li>Assumptions of Rankines Theory: - <ol> <li>The soil is semi infinite, homogenous, dry and cohesion less.</li> <li>The soil element is in the state of plastic equilibrium.</li> <li>The ground surface is plane which may be horizontal.</li> <li>The back of is vertical and smooth.</li> <li>The wall yield about the base thus satisfies deformation condition for plastic equilibrium.</li> <li>Rankines theory for cohesion less backfill: - <ol> <li>Dry Backfill</li> <li>Submerged Backfill</li> <li>Surcharge for horizontal Plane surface.</li> </ol> </li> <li>b) Submerged backfill the land behind the wall is saturated with water and lateral pressure is having pressure due to submerged unit weight and water.</li> <li>If water is present on both side of wall then the pressure is not to be considered.</li> <li>c) Surcharge for horizontal plane pressure: - <ol> <li>The back fill is horizontal and carries a surcharge of uniform intensity as per unit area.</li> <li>The pressure at the base of retaining wall consists of pressure made of unit weight of backfill and pressure due to uniform surcharge.</li> </ol> </li> </ol></li></ul>