









Length of new bridge =  $4 \times 50 = 200$  m

02 M

Approximate cost of new bridge =  $200 \times 75757.58$   
= Rs. 15151516.

02 M

Q.2 c) The formation level of road at starting point is 470.00 m. The road surface shall be falling gradient line of 1 to 60 formation width of road is 12 m. Side slope 1:2 in embankment and 1:1.5 in cutting. .  
Assume there is no cross slope to the ground.

Chainage in M	0	30	60	90	120	150
R.L. of G.L. in m	465.00	467.20	468.10	468.20	469.70	469.00

Calculate the quantity of earthwork. For road using Mean Sectional Area Method.

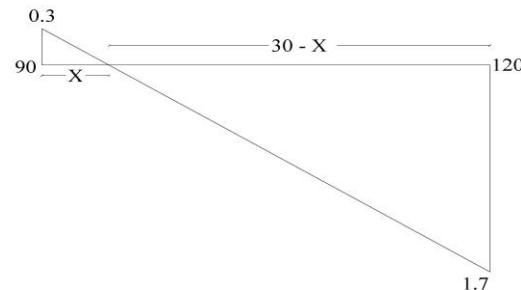
Ans

Fall in gradient 1:60

Fall in formation level for each 30 m =  $30 / 60 = 0.5$  m.

Chainage in M	0	30	60	90	120	150
R.L. of G.L. in m	465.00	467.20	468.10	468.20	469.70	469.00
Formation level	470.0	469.5	469.0	468.5	468.0	467.5
Height of embankment	5.0	2.3	0.9	0.3	--	--
Depth of cutting	--	--	--	--	1.7	1.5

02 M



$$\begin{aligned} X / 0.3 &= (30 - X) / 1.7 \\ 1.7X &= 9 - 0.3X \\ 2X &= 9 \\ X &= 4.5 \end{aligned}$$

No banking and no cutting at section =  $90 + 4.5 = 94.5$  m

01 M

Chainage	Depth 'd'	Area		Mean area		L	Volume		
		Banking S=2	Cutting S=1.5	Banking	Cutting		Banking	Cutting	
0	5.0	110	--	--	--	--	--	--	
30	2.3	38.18	--	74.09	--	30	2222.7	--	
60	0.9	12.42	--	25.3	--	30	759	--	
90	0.3	3.78	--	8.1	--	30	243	--	
94.5	0	0	--	1.89	--	4.5	8.505	--	
120	-1.7	--	24.735	--	-12.3675	25.5	--	315.37	
150	-1.5	--	21.375	--	-23.055	30	--	691.65	
Total							3233.205	1007.02	

04 M

Earth work in banking = 3233.205 Cu.m.

Earth work in cutting = 1007.02 Cu.m.

01 M

Q.3 a) Attempt any FOUR of the following:  
How will you prepare estimate for irrigation canal?  
Estimate for irrigation canal.  
The unit to be adopted for finding out the approximate estimate of irrigation canal , is one



		<p>of the following</p> <p>a) Area of land under command of canal</p> <p>b) Per Km length.</p> <p>In the first case, the area under the command of irrigation canal is worked out in hectares. Knowing the cost of similar project, suitable amount per hectare is decided. The approximate estimate is calculated by multiplying the area under the command to per hectare cost of canal.</p> <p>In second case cost per Km length is calculated from the similar units, constructed previously.</p> <p>The approximate cost of proposed canal is calculated by multiplying the length of proposed canal to cost per km length of canal.</p> <p>An amount of contingencies, normally 10% is added to the cost of project. For overheads, 10% cost of approximate estimate is included in the estimate.</p> <p>At last cost for land acquisition, normally 12% is added to get total approximate estimated cost of a project.</p>	<p>01 M</p> <p>02 M</p> <p>01 M</p>										
Q.3	b) Ans	<p>Describe D.S.R. State its uses.</p> <p>D.S.R.: - A list of rates of various items is prepared to facilitate preparation of estimate by government bodies like Public Works Department. As the rates vary from place to place, Maharashtra Government publishes list of rates as per districts. These rates are in the form of printed booklet and called as District Schedule of Rates (DSR).</p> <p>This booklet is revised every year because of changes in cost of labor, material every year.</p> <p>It includes Completed rates, per unit cost of item of work and Labor rates.</p> <p>Per unit cost of item includes cost of material, cost of labor, transportation charges, storage of material, charges for tools machineries and plants etc.</p> <p>Labor rates include charges to be paid to head mason, mazdoor, coolie etc. depending on the category of labor.</p> <p>It also includes initial lead and lift and separate charges are applicable for more lead and lift. Similarly the rates are applicable to ground floor only and they are increased for each upper floor.</p> <p>Uses of DSR: a) The work carried out by the department is estimated according to DSR. b) Rates shown in DSR keep check on rates quoted by the contractor. c) If rates of contractor differ much with rates in DSR, tender may be rejected.</p>	<p>01 M</p> <p>02 M</p> <p>01 M</p>										
Q.3	c) Ans	<p>Give the hire charges for following machinery/equipment.</p> <p>(i) Concrete mixer      (ii) Dumper      (iii) Vibrator      (iv) JCB</p> <table border="1" style="margin-left: auto; margin-right: auto;"> <thead> <tr> <th>Machinery/Equipment</th> <th>Hire charges</th> </tr> </thead> <tbody> <tr> <td>1) Concrete Mixer</td> <td>Rs. 600 to Rs. 800 per day</td> </tr> <tr> <td>2) Dumper</td> <td>Rs. 1000 to Rs. 1200 per day</td> </tr> <tr> <td>3) Vibrator</td> <td>Rs. 400 to Rs. 500 per day</td> </tr> <tr> <td>4) JCB</td> <td>Rs. 800 to Rs. 1000 per hour</td> </tr> </tbody> </table> <p><b>Note:- Hire charges may vary from place to place.</b></p>	Machinery/Equipment	Hire charges	1) Concrete Mixer	Rs. 600 to Rs. 800 per day	2) Dumper	Rs. 1000 to Rs. 1200 per day	3) Vibrator	Rs. 400 to Rs. 500 per day	4) JCB	Rs. 800 to Rs. 1000 per hour	<p>01 M for each</p>
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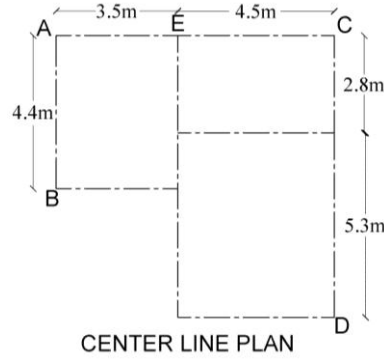


Q.3	d) Ans	<p>State the desired accuracy in taking measurement of work as per IS 1200.</p> <p>To achieve the desired accuracy in measurements, following points must be observed.</p> <p>1) Dimensions shall be measured to the nearest 0.01m except</p> <p>a) Thickness of slab measured nearest to 0.005m</p> <p>b) Wood work is to be measured nearest to 0.002m</p> <p>c) Reinforcement , to the nearest 0.005m</p> <p>d) Thickness of roadwork less than 200mm is measured nearest to 0.005m.</p> <p>The tolerances in measurements are</p> <p>a) For volumes ----- 0.01 cu.m</p> <p>b) For areas -----0.01 sq.m</p> <p>c) For lengths ----- 0.01 rmt</p> <p>d) For weights -----0.001 ton or 1kg.</p> <p>Fraction less than one half is neglected.</p> <p>Fraction equal to one half or more than one half is considered</p>	02 M  02 M
Q.3	e) Ans	<p>Explain the long wall and short wall method for taking out quantities.</p> <p>In this method longer walls in building in one direction are consider as long wall and it is measured out to out. Walls in perpendicular direction of long walls, are consider as short walls and measured in to in for a particular layer of work. This is most practical method as it can be used under all circumstances. Following steps are involved in this method.</p> <p>1) Foundation plan showing centre line with all dimensions. Centre to centre length is calculated by adding half width of each cross wall to inner dimensions of a room.</p> <p>2) Group the walls as long walls and short walls. Measure the length of long wall for an item using equation, length of long wall = c/c length of long wall + width of item at that layer.</p> <p>3) Measure the length of short wall for an item using equation, length of short wall = c/c length of short wall - width of item at that layer.</p> <p>4) Multiply number of walls, length, breadth and depth to get the quantity of item.</p> <p>This method is simple, quick and accurate. Method is also known as PWD method. At every layer from foundation to superstructure, length of long wall decreases gradually and length of short wall increases.</p>	02 M  02 M
Q.3	f) Ans	<p>State purpose of supplementary estimate. Give one example.</p> <p>1) During the execution of project, certain new items or additional works crop up to supplement the original project. Under such circumstances, it becomes necessary to prepare supplementary estimate.</p> <p>2) Supplementary estimate is prepared for covering the estimate of sub-work of a project, which is considered necessary for full development of project.</p> <p>3) Sometimes changes due to material deviation of a structural nature from the original approved design are necessary when the work is in progress. Then for all such items supplementary estimate is prepared.</p> <p>Example: If in a bed room of Bungalow of executive engineer, attached toilet is not provided in original project and then it is decided to construct attached toilet, then supplementary estimate is necessary.</p>	03 M  01 M



Q.4 a) Work out quantities of the following any THREE items of work from Figure No.1.  
 (i) Excavation for foundation (ii) Brick work in super structure in c.m. (1 : 6)  
 (iii) Internal plaster in c.m. (1 : 4) (iv) R.C.C. slab (1 : 2 : 4)

Ans C/C distance AB = 4.1 + 0.3 = 4.4 m.  
 C/C distance CD = 7.5 + 0.6 = 8.1 m  
 C/C distance AE = 3.2 + 0.3 = 3.5 m  
 C/C distance EC = 4.2 + 0.3 = 4.5 m



Measurement Sheet

S.N.	Description	No	L	B	D/H	Qty	Total Qty	
1)	Excavation for foundation	1	5.4	1.0	1.15	6.21 m <sup>3</sup>	44.965m <sup>3</sup>	
	AB = 4.4+1.0=5.4	2	9.1	1.0	1.15	20.93m <sup>3</sup>		
	CD= 8.1+1.0=9.1	2	2.5	1.0	1.15	5.75 m <sup>3</sup>		
	AE= 3.5-1.0=2.5	3	3.5	1.0	1.15	12.075m <sup>3</sup>		
	EC =4.5-1.0=3.5							
<b>OR</b> by center line method								
Total center line length = 41.1 m								
Effective center line length =		1	39.1	1.0	1.15	44.965m <sup>3</sup>	44.965m <sup>3</sup>	
41.1 – 4 x 1.0/2 = 39.1 m.								
2)	Brick work in superstructure in CM (1:6)	1	4.7	0.3	3.0	4.23 m <sup>3</sup>	31.104m <sup>3</sup>	
	AB = 4.4+0.3=4.7	2	8.4	0.3	3.0	15.12 m <sup>3</sup>		
	CD= 8.1+0.3=8.4	2	3.2	0.3	3.0	5.76 m <sup>3</sup>		
	AE= 3.5-0.3=3.2	3	4.2	0.3	3.0	11.34 m <sup>3</sup>		
	EC =4.5-0.3=4.2							
	Deductions							
	Door D	3	1.0	0.3	2.1	-1.89 m <sup>3</sup>		
	Windows W1	5	1.2	0.3	1.2	-2.16 m <sup>3</sup>		
	Windows W	2	1.8	0.3	1.2	-1.296m <sup>3</sup>		
	<b>OR</b> by center line method							
Total center line length = 41.1 m								
Effective center line length =		1	40.5	0.3	3.0	36.45 m <sup>3</sup>		
41.1 – 4 x 0.3/2 = 40.5 m.								
Deductions						-5.346m <sup>3</sup>	31.104m <sup>3</sup>	
3)	Internal plaster in CM (1:4)							
	Ceiling							
	Bed room	1	3.2		4.1	13.12 m <sup>2</sup>		
	Kitchen	1	4.2		2.5	10.5 m <sup>2</sup>		
	Living	1	4.2		5.0	21.0 m <sup>2</sup>		
Internal faces of wall								

Any  
Three  
04 M  
each



**MAHARASHTRA STATE BOARD OF TECHNICAL EDUCATION**  
(Autonomous)  
(ISO/IEC - 27001 - 2013 Certified)

		Bed room	2	3.2		3.0	19.2 m <sup>2</sup>		
			2	4.1		3.0	24.6 m <sup>2</sup>		
		Kitchen	2	4.2		3.0	25.2 m <sup>2</sup>		
			2	2.5		3.0	15.0 m <sup>2</sup>		
		Living	2	4.2		3.0	25.2 m <sup>2</sup>		
			2	5.0		3.0	30.0 m <sup>2</sup>		
		Deductions							
		Doors	2.5	1.0		2.1	-5.25 m <sup>2</sup>		
		Windows W1	2.5	1.2		1.2	-3.60 m <sup>2</sup>		
		Windows W	1	1.8		1.2	-2.16 m <sup>2</sup>		172.81m <sup>2</sup>
4)	R.C.C. Slab (1:2:4)	1	8.4	4.8	0.15	6.048 m <sup>3</sup>			
		1	4.7	3.5	0.15	2.468 m <sup>3</sup>		8.516 m <sup>3</sup>	
Q.4	b) (i) Ans	<p>Attempt any ONE of the following:</p> <p>Find out the quantities of cement, sand and aggregate for R.C.C. 1:2:4 work of 25 cu.m quantity.</p> <p>Quantities of cement, sand and aggregate.</p> <p>Wet volume of concrete given is 25 cu.m</p> <p>Add 52% more for voids and wastage to get dry volume. -----</p> <p>Dry volume = 25 + 25 (52/100) = 38 cu.m -----</p> <p>Concrete is in proportion 1:2:4 , hence</p> <p>Quantity of cement = [dry volume / (1+2+4)] x part of cement</p> <p style="padding-left: 40px;">= (38/7) x 1 = 5.43cu.m -----</p> <p style="padding-left: 40px;">Number of bags= 5.43 / 0.035 = 155.1 bags = 155 bags OR 156 bags. -----</p> <p>Quantity of sand = (38 / 7) x 2 = 10.86 cu.m -----</p> <p>Quantity of aggregate = (38 / 7) x 4 = 21.71 cu.m -----</p> <p><b>Note: Someone may take volume of 1 bag of cement as 0.034 m<sup>3</sup>.</b></p>						01 M 01 M  01 M 01 M 01 M 01 M	





**Q.4**    **b)(ii)**    A R.C.C. simply supported beam of size 295 mm x 645 mm is reinforced with 4 Nos. of 20 mm diameters main bars are placed in one row and two bent up. Two Anchor bars of 12 mm diameter are provided at top. 8 mm diameters stirrups are provided at 140 mm c/c. The overall beam length is 6 m. Calculate the total quantities of the steel required. Show bar bending schedules.

**Ans**    Data given: Beam size 295mm x 645 mm  
Length of beam = 6m  
Main bars: 4 no of 20 mm diameter out of which two bent up.  
Anchor bars at top: two anchor bars of 12 mm dia.  
Stirrups : 8 mm dia. provided at 140 mm c/c

Assume cover to the reinforcement 25 mm from all sides.

Bars are bent up at 45°

180° Hook is provided on both sides

a) Calculate length of one straight bar with 180° hook on both sides.

$$L = 6000 - 2 \times 25 + 2 \times 9 \times 20 = 6310 \text{ mm} \\ = 6.31\text{m} \text{ -----}$$

01 M

b) Length of bent up bars with 180° hook on both sides.

$$L = 6000 - 2 \times 25 + 2 \times 9 \times 20 + 2 \times 0.42 \times (645 - 50) = 6809.80\text{mm} \\ = 6.81\text{m} \text{ -----}$$

01 M

c) Length of anchor bar at top with 180° hook on both sides.

$$L = 6000 - 2 \times 25 + 2 \times 9 \times 12 = 6166 \text{ mm} \\ = 6.17 \text{ m} \text{ -----}$$

01 M

d) length of 2 legged stirrup

$$L = 2 \times (295 - 2 \times 25) + 2 \times (645 - 2 \times 25) + 24 \times 8 = 1872\text{mm} = 1.87 \text{ m}$$

$$\text{No of stirrups} = [(6000 - 2 \times 25) / 140] + 1 = 43.5 = 44 \text{ nos. -----}$$

01 M

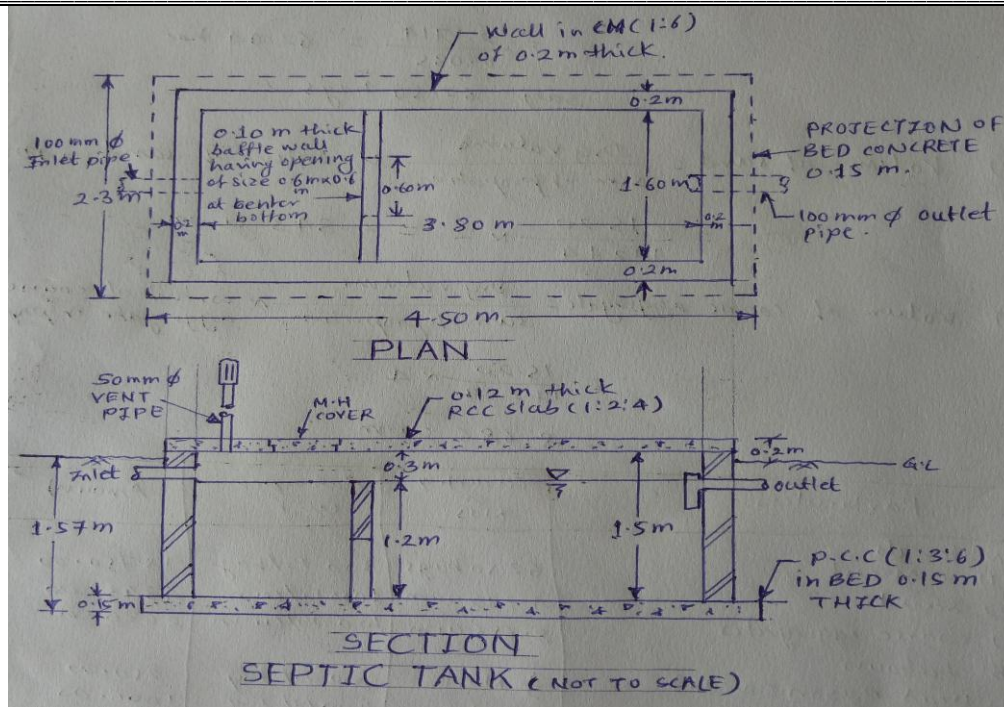
Bar bending schedule:

S.N	Particulars	Shape of bar	No of bars	Dia.of Bar(mm)	Total length (m)	Wt of Bar in kg/m	Total wt In kg
1)	Top anchor bars with hook on both sides,		2	12mm	12.34m	0.89	10.98 kg
2)	Two straight bars at bottom with hook on both sides		2	20mm	12.62m	2.47	31.17kg
3)	Two bent up bars at 450		2	20mm	13.62m	2.47	33.64kg.
4)	Two legged stirrups		44	8 mm	82.28m	0.39	32.09kg.
						<b>Total</b>	<b>107.88kg.</b>

02 M



Q.5	a) Ans	<p>Attempt any TWO of the following:</p> <p>Prepare rate analysis for plain cement concrete of grade M15 (1:2:4). Assume Wet volume of Concrete = 10 Cu. m.</p> <p>(A) Calculation of materials :</p> <p>Therefore, Dry volume of concrete = 52 % more of wet concrete = <math>10 + ((52/100) \times 10) = 15.20</math> cu. m. PCC Grade M15 having proportion 1:2:4</p> <p>(1) Volume of cement = (dry volume/sum of proportion) x part of cement = <math>(15.20/(1+2+4)) \times 1 = 2.1714</math> cu. m. Therefore no. of cement bags = volume of cement / vol. of cem. Per bag = <math>2.1714 / 0.035 = 62.04</math> say 62.50 bags.</p> <p><b>Note: Someone may take volume of 1 bag of cement as <math>0.034 \text{ m}^3</math>.</b></p> <p>(2) Volume of sand = (dry volume/ sum of proportion) x part of sand. = <math>(15.20/(1+2+4)) \times 2 = 4.343</math> cu. m.</p> <p>(3) Volume of coarse aggregate. = (dry volume/ sum of proportion) x part of agg. = <math>(15.20/(1+2+4)) \times 4 = 8.686</math> cu. m.</p> <p>(B)Table for rate analysis for 10 Cu. m.</p> <table border="1" data-bbox="228 808 1263 1633"> <thead> <tr> <th>Particulars</th> <th>Quantity</th> <th>Rate per unit</th> <th>Unit of mesurts.</th> <th>Amount (Rs.)</th> </tr> </thead> <tbody> <tr> <td>(A) Material :</td> <td></td> <td></td> <td></td> <td></td> </tr> <tr> <td>Cement</td> <td>62.50 bags</td> <td>Rs. 330</td> <td>bag</td> <td>20625.00</td> </tr> <tr> <td>Sand</td> <td>4.343 cu. m.</td> <td>Rs. 800</td> <td>Cu. m.</td> <td>3474.40</td> </tr> <tr> <td>Coarse Aggregate</td> <td>8.686 cu. m.</td> <td>Rs. 800</td> <td>Cu. m.</td> <td>6948.80</td> </tr> <tr> <td>(B) Labour :</td> <td></td> <td></td> <td></td> <td></td> </tr> <tr> <td>Head mason (Mistri)</td> <td>1/2 no.</td> <td>Rs. 500</td> <td>day</td> <td>250.00</td> </tr> <tr> <td>Mason</td> <td>2.5 no.</td> <td>Rs. 400</td> <td>day</td> <td>1000.00</td> </tr> <tr> <td>Male Mazdoor</td> <td>12 no.</td> <td>Rs. 300</td> <td>day</td> <td>3600.00</td> </tr> <tr> <td>Female Mazdoor</td> <td>18 no.</td> <td>Rs. 250</td> <td>day</td> <td>4500.00</td> </tr> <tr> <td>Bhisti (including curing)</td> <td>6 no.</td> <td>Rs. 250</td> <td>day</td> <td>1500.00</td> </tr> <tr> <td>Forms etc. (as per req.)</td> <td>Lump sum</td> <td>1800 L.S.</td> <td>--</td> <td>1800.00</td> </tr> <tr> <td>Sundries T and P etc.</td> <td>Lump sum</td> <td>300 L.S.</td> <td>--</td> <td>300.00</td> </tr> <tr> <td colspan="4">Total of Material and Labour</td> <td>43998.20</td> </tr> <tr> <td colspan="4">Add 1.5 % water charges</td> <td>659.97</td> </tr> <tr> <td colspan="4">Add 10 % contractors profit</td> <td>4399.82</td> </tr> <tr> <td colspan="4">Grand Total (Rate per 10 Cu. m.)</td> <td>49057.99</td> </tr> <tr> <td colspan="4">Rate per Cu. m.</td> <td>4905.79</td> </tr> <tr> <td colspan="4">Say</td> <td>Rs.4906.00</td> </tr> </tbody> </table> <p><b>(Note: Assumption can be made by understanding of student. Rate may vary from place to place.)</b></p>	Particulars	Quantity	Rate per unit	Unit of mesurts.	Amount (Rs.)	(A) Material :					Cement	62.50 bags	Rs. 330	bag	20625.00	Sand	4.343 cu. m.	Rs. 800	Cu. m.	3474.40	Coarse Aggregate	8.686 cu. m.	Rs. 800	Cu. m.	6948.80	(B) Labour :					Head mason (Mistri)	1/2 no.	Rs. 500	day	250.00	Mason	2.5 no.	Rs. 400	day	1000.00	Male Mazdoor	12 no.	Rs. 300	day	3600.00	Female Mazdoor	18 no.	Rs. 250	day	4500.00	Bhisti (including curing)	6 no.	Rs. 250	day	1500.00	Forms etc. (as per req.)	Lump sum	1800 L.S.	--	1800.00	Sundries T and P etc.	Lump sum	300 L.S.	--	300.00	Total of Material and Labour				43998.20	Add 1.5 % water charges				659.97	Add 10 % contractors profit				4399.82	Grand Total (Rate per 10 Cu. m.)				49057.99	Rate per Cu. m.				4905.79	Say				Rs.4906.00	01 M  01 M  01 M  01 M  04 Marks for Table and values.
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Q.5	b)          Ans	<p>Workout the quantity of the following hems. For septic tank having internal size 1.6 m x 3.8 m having height 1.5 m. The top of slab of septic tank is 20 cm. above G.L.</p> <p>(i) Earthwork in Excavation                      (ii) P.C.C. (1:3:6) 15 cm thick. (iii) B.B. Masonry in C.M. (1:6) of 200 mm thick. (iv) R.C.C. slab (1:2:4) on septic tank 12 cm thick.</p> <p>First of all , draw the plan and sectional elevation of Septic tank from the given data</p>																																																																																																



02 Marks for Figure

Sr. No.	Item of work	No.	Length (m.)	Width (m.)	Height / Depth (m.)	Qty.
1.	Excavation					
	(a) up to 1.5 m depth	1	4.50	2.30	1.5	15.53 cu.m.
	(b) from 1.5 m to 3.0 m depth	1	4.50	2.30	0.07	0.72 cu.m.
	Total Earthwork in Excavation					16.25 cu.m.
2.	P.C.C. (1:3:6) 15 cm thick	1	4.50	2.30	0.15	1.55 cu.m.
3.	B.B.Masonry in C.M. (1:6) of 200 mm thick					
	(a) Long walls	2	4.20	0.20	1.50	2.52 cu.m.
	(b) Short walls	2	1.60	0.20	1.50	0.96 cu.m.
	(c) baffle wall	1	1.60	0.10	1.20	0.19 cu.m.
	Deduction for opening in baffle wall	1	0.600	0.10	0.60	(-)0.04 cu.m.
	Total Quantity of B.B. Masonry					3.63 cu.m.
4.	12 mm thk. RCC slab with manhole cover	1	4.20	2.00	0.012	0.10 cu.m.

01 M

01 M

02 M

02 M

Q.5  
c)  
Ans

Prepare the rate analysis for U.C.R. masonry in c.m. (1:4) in foundation.

Assume Volume of masonry = 10 Cu. m.

(A) Calculation of materials :

Therefore, Dry volume of cement mortar = 42% of volume of masonry  
=  $(42/100) \times 10 = 4.20$  cu. m.

(1) Volume of cement =  $(\text{dry volume} / \text{sum of proportion}) \times \text{part of cement}$

01 M



		$= (4.20/(1+4)) \times 1 = 0.84 \text{ cu. m.}$ <p>Therefore no. of cement bags = volume of cement / vol. of cement bag  <math display="block">= 0.84 / 0.035 = 24 \text{ bags.}</math></p> <p>(2) Volume of sand = (dry volume/ sum of proportion) x part of sand.  <math display="block">= (4.20/(1+4)) \times 4 = 3.36 \text{ cu. m.}</math></p> <p>(3) Volume of Stone = 25 % more of volume of masonry.  <math display="block">= ((25/100) \times 10) + 10 = 12.50 \text{ cu. m.}</math></p> <p>(B) Table for rate analysis for 10 Cu. m.</p> <table border="1"> <thead> <tr> <th>Particulars</th> <th>Quantity</th> <th>Rate per unit</th> <th>Unit of mesurts.</th> <th>Amount (Rs.)</th> </tr> </thead> <tbody> <tr> <td>(A) Material :</td> <td></td> <td></td> <td></td> <td></td> </tr> <tr> <td>Cement</td> <td>24 bags</td> <td>Rs. 330</td> <td>bag</td> <td>7920.00</td> </tr> <tr> <td>Sand</td> <td>3.36 cu. m.</td> <td>Rs. 800</td> <td>Cu. m.</td> <td>2688.00</td> </tr> <tr> <td>Stone including through bond stone and wastage</td> <td>12.50 cu. m.</td> <td>Rs. 1100</td> <td>Cu. m.</td> <td>13750.00</td> </tr> <tr> <td>(B) Labour :</td> <td></td> <td></td> <td></td> <td></td> </tr> <tr> <td>Head mason (Mistri)</td> <td>1/2 no.</td> <td>Rs. 500</td> <td>day</td> <td>250.00</td> </tr> <tr> <td>Mason</td> <td>12 no.</td> <td>Rs. 400</td> <td>day</td> <td>4800.00</td> </tr> <tr> <td>Male Mazdoor</td> <td>10 no.</td> <td>Rs. 300</td> <td>day</td> <td>3000.00</td> </tr> <tr> <td>Female Mazdoor</td> <td>10 no.</td> <td>Rs. 250</td> <td>day</td> <td>2500.00</td> </tr> <tr> <td>Bhisti</td> <td>1.5 no.</td> <td>Rs. 250</td> <td>day</td> <td>375.00</td> </tr> <tr> <td>Scaffolding</td> <td>Lump sum</td> <td>400 L.S.</td> <td>--</td> <td>400.00</td> </tr> <tr> <td>Sundries T and P etc.</td> <td>Lump sum</td> <td>300 L.S.</td> <td>--</td> <td>300.00</td> </tr> <tr> <td colspan="4" style="text-align: right;">Total of Material and Labour</td> <td>35983.00</td> </tr> <tr> <td colspan="4" style="text-align: right;">Add 1.5 % water charges</td> <td>539.75</td> </tr> <tr> <td colspan="4" style="text-align: right;">Add 10 % contractors profit</td> <td>3598.30</td> </tr> <tr> <td colspan="4" style="text-align: right;">Grand Total (Rate per 10 Cu. m.)</td> <td>40121.05</td> </tr> <tr> <td colspan="4" style="text-align: right;">Rate per Cu. m.</td> <td>4012.11</td> </tr> <tr> <td colspan="4" style="text-align: right;">Say</td> <td>Rs.4012.00</td> </tr> </tbody> </table> <p>(Note : Assumption can be made by understanding of student. Rate may vary from place to place.)</p>	Particulars	Quantity	Rate per unit	Unit of mesurts.	Amount (Rs.)	(A) Material :					Cement	24 bags	Rs. 330	bag	7920.00	Sand	3.36 cu. m.	Rs. 800	Cu. m.	2688.00	Stone including through bond stone and wastage	12.50 cu. m.	Rs. 1100	Cu. m.	13750.00	(B) Labour :					Head mason (Mistri)	1/2 no.	Rs. 500	day	250.00	Mason	12 no.	Rs. 400	day	4800.00	Male Mazdoor	10 no.	Rs. 300	day	3000.00	Female Mazdoor	10 no.	Rs. 250	day	2500.00	Bhisti	1.5 no.	Rs. 250	day	375.00	Scaffolding	Lump sum	400 L.S.	--	400.00	Sundries T and P etc.	Lump sum	300 L.S.	--	300.00	Total of Material and Labour				35983.00	Add 1.5 % water charges				539.75	Add 10 % contractors profit				3598.30	Grand Total (Rate per 10 Cu. m.)				40121.05	Rate per Cu. m.				4012.11	Say				Rs.4012.00	<p>01 M</p> <p>01 M</p> <p>01 M</p> <p>04 M for Table and values</p>
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Q.6	a) Ans	<p>Attempt any FOUR of the following:</p> <p>Defme 'Task work'. Enlist any four factors affecting task work.</p> <p>Task Work: The capacity of doing work by an artisan or skilled labour in the form of quantity of work per day is known as the Task-Work or Out-turn of the labour.</p> <ul style="list-style-type: none"> <li>• Factors affecting task work of the labour:</li> </ul> <p>Task work of a skilled labour depends upon the following factors:</p> <ol style="list-style-type: none"> <li>(1) Nature, Size, Height, situation, location, climatic condition, techniques adopted and wages paid etc.</li> <li>(2) Availability of skilled labour.</li> <li>(3) A well-organized work.</li> <li>(4) Job satisfaction or working conditions.</li> <li>(5) Allotment of piece of work.</li> </ol>	<p>02 M</p> <p>Any four (1/2 M each)</p>																																																																																															

Q.6 b) State the names of software that are used for preparation of detailed estimate of building work.

Ans Following are the names of software that are used for preparation of detailed estimate of building work.

1. Sage Estimating
2. CoConstruct
3. Buildertrend
4. Clear Estimates
5. MEP Estimating
6. Corecon
7. Sigma
8. Esticom
9. Estimator
10. STACK
11. Build-Quant
12. Build-Master
13. Civil estimator
14. Turbo Bid
15. Intelli Bid
16. Pro Est
17. B2W (BID2Win)

**(Note :** The names of software may vary with student knowledge. Therefore marks should be given with respect to that.)

Any eight  
(1/2 M each)

Q.6 c) What are the different methods used for calculation of earthwork quantities for a road and canal? Explain any one.

Ans The different methods used for calculation of earthwork quantities for road and canal are as follows:

1. Mid-sectional area method
2. Mean Sectional area method
3. Prismoidal formula method
4. Trapezoildal formula method

**(1) Mid-sectional are method:** In this method, the mid-section area is calculated by dividing the trapezoidal cross-section of Road/Canal into rectangle and two triangles and then this mid-section area is multiplied by the length of the section to get quantity of earthwork as given below:

Area of mid section = Area of rectangular portion + area of two triangular portion

$$= Bd_m + \frac{1}{2}sd_m^2 + \frac{1}{2}sd_m^2 = Bd_m + sd_m^2$$

∴ Quantity of earthwork =  $(Bd_m + sd_m^2) \times L$

General,  $Q = (Bd + sd^2) \times L$ , where d stands for mean height or depth.

The quantities of earthwork may be calculated in a tabular form as below :-

Stations or Chain-age	Depth or Height	Mean Depth or Height "d"	Area of central portion Bd	Area of sides $Sd^2$	Total Sectional Area $Bd+sd^2$	Length between stations L	Quantity $(Bd + sd^2) \times L$	
							Embankment	Cutting

Fig. 7-4

02 M

02 M  
(For any one method)

**(2) Mean Sectional area method :** In this method, cross sectional area at two ends of section is calculated and then mean of these two is multiplied by length of section to get volume of earthwork.

Sectional area at one end  $A_1 = Bd_1 + sd_1^2$ , sectional area at the other end  $A_2 = Bd_2 + sd_2^2$ ,  $d_1$  and  $d_2$  are the heights or depth at the two ends.

The mean sectional area  $A = \frac{A_1 + A_2}{2}$ , Quantity  $Q = \frac{A_1 + A_2}{2} \times \text{Length}$ .

The quantities of earthwork may be calculated in a tabular form as given below :-

Stations or Chainage	Height or Depth "d"	Area of central portion $Bd$	Area of sides $Sd^2$	Total Sectional Area $Bd + Sd^2$	Mean Sectional Area	Length between station L	Quantity $(Bd + sd^2) \times L$	
							Embankment	Cutting

**(3) Prismoidal Formula Method:** In this method, the cross sectional area at the two ends of a portion of embankment is calculated and then mid sectional area is also calculated. The quantity or volume can be calculated by following formula:

$$\text{Quantity or volume} = (L/6) \times (A_1 + A_2 + A_m)$$

Where  $A_1$  and  $A_2$  are the cross sectional areas at the two ends of a portion of embankment of road or canal of length  $L$ ,  $A_m$  is the mid-sectional area.

Let  $d_1$  and  $d_2$  be the heights of banks at the two ends, and  $d_m$  be the mean height at the mid-section,  $B$  be the formation width and  $S:1$  be the side slope. Then

$$\begin{aligned} \text{Quantity} &= \frac{L}{6} (A_1 + A_2 + 4A_m) \\ &= \frac{L}{6} [(Bd_1 + sd_1^2) + (Bd_2 + sd_2^2) + 4 \{ B(\frac{d_1 + d_2}{2}) + s(\frac{d_1 + d_2}{2})^2 \}] \\ &= \frac{L}{6} [(Bd_1 + Bd_2 + 4 \frac{Bd_1}{2} + 4 \frac{Bd_2}{2}) + sd_1^2 + sd_2^2 + 4s \frac{d_1^2 + d_2^2 + 2d_1d_2}{4}] \\ &= \frac{L}{6} [(3Bd_1 + 3Bd_2) + 2sd_1^2 + 2sd_2^2 + 2sd_1d_2] \\ &= \frac{3BL}{6} (d_1 + d_2) + \frac{2LS}{6} (d_1^2 + d_2^2 + d_1d_2) \\ &= \frac{BL}{2} (d_1 + d_2) + \frac{LS}{3} (d_1^2 + d_2^2 + d_1d_2) \\ &= \{ B(\frac{d_1 + d_2}{2}) + s(\frac{d_1^2 + d_2^2 + 2d_1d_2}{3}) \} \times L \\ &= [\text{Sec. Area of central portion} + \text{Sec. Area of side slope portions}] \times \text{Length}. \end{aligned}$$

The same is also applicable for cutting.

**(4) Trapezoidal Formula Method:** When a series of cross-sectional areas calculated at equidistant points, the volume may be worked out by Trapezoidal formula.

Let  $A_0, A_1, A_2, A_3, A_4, \dots, A_n$  are the areas of cross-sections.

$D$  = Distance between the section:  $V$  = volume of cutting or banking.

$$\begin{aligned} \text{Volume by Trapezoidal Formula Method} &= \\ V &= \frac{D}{2} \{ A_0 + 2A_1 + 2A_2 + 2A_3 + \dots + 2A_{n-1} + A_n \} \\ &= D \{ \frac{A_0 + A_n}{2} + A_1 + A_2 + A_3 + \dots + A_{n-1} \} \end{aligned}$$

Q.6 d) Define rate analysis and state the factors affecting rate analysis.

Ans Rate analysis : The determination of rate per unit of a particular item of work, from the cost of quantities of materials, the cost of labourers and other miscellaneous petty expenses require for its completion is known as the rate analysis.



\*Factors affecting the rate analysis :-

The factors which affect the rate analysis of an item can be broadly divided into following :

(1) Major Factors and (2) Minor Factors

**(1) Major factors :** The are mainly two factors on which the rate of an item depends,-----

**(i) Materials and (ii) Labour.**

(i) Materials :-

The quantities of various materials required for the construction of an item can be easily worked out by knowing the specification of that item.

(ii) Labour :-

The labour force will be necessary to arrange the materials in a proper way so that the item can be completed.

**(2) Minor Factors :-**

(i) Special equipment: - If the execution of an item requires the use of some special equipment ort plant, the cost of using such special equipment on the rental basis should be included in the rate analysis of that item.

(ii) Place of work :- The site of work will also have some effect on the rate of an item under certain conditions. If it is too far, more amount will have to be spent on carting. This will increase the cost of transportation of the materials and consequently, the rates of the items are to be modified.

(iii) Nature of work :- If the work consists if large quantities of the items, the rates may be less and vice versa.

(iv) Conditions of contract :- If the condition of contract are very stiff, the rates of various items will be high and vice versa.

(v) Profit of the contractor :- The usual percentage of the profit of the contractor is TEN. But if it is more or less, the rate of the item will be correspondingly affected.

(vi) Specifications :- If the specifications of work provide for rigid type tolerances and superior quality turn out, the rates will be on the higher side.

(vii) Site conditions :- If the site conditions are such that difficulties will be experienced during execution of work, such as foundations involving water troubles, th0e rates will be on the higher side. On the other hand, if site conditions are ideally suited for the construction activities, the contractor may quote slightly lower rates.

(viii) Miscellaneous :- The other remaining miscellaneous factors affecting rates of items include time of completion of the project, climatic conditions, reputation of the contracting firm, discipline of the organization, etc.

01 M

any four  
1/2 M  
each



Q.6

e)

Calculate the quantities of following items of work for a circular community well as shown in Figure No.2.

(i) Excavation in Soft Murum. (ii) R.C.C. Ring Beam quantity of concrete.

Ans

From the Figure no. 2

Qty. of Excavation and concrete is calculated in Table below:

Sr. No.	Item of work	Nos.	Length	width	depth / thk.	Quantity
			OR Area			
<b>(i) Excavation in soft murum</b>						
1	i) up to 1.5 m depth	1	$((\pi/4) \times 5.20^2)$ sq. m.		1.5 m	31.86 cu. m.
2	ii) 1.5m to 3.0 m depth	1	$((\pi/4) \times 5.20^2)$ sq. m.		1.5 m	31.86 cu. m.
3	iii) 3.0m to 4.5 m depth	1	$((\pi/4) \times 5.20^2)$ sq. m.		1.5 m	31.86 cu. .
4	iv) 4.5m to 6.0 m depth	1	$((\pi/4) \times 5.20^2)$ sq. m.		1.5 m	31.86 cu. m.
5	iv) 6.0m to 7.5 m depth	1	$((\pi/4) \times 5.20^2)$ sq. m.		1.5 m	31.86 cu. m.
6	iv) 7.5m to 8.5 m depth	1	$((\pi/4) \times 5.20^2)$ sq. m.		1.0 m	21.24 cu. m.
<b>Total excavation of soft rock</b>						<b>180.54 cu. m.</b>
<b>(ii) R.C.C. Ring beam (RCC M20)</b>						
The Ring Beam has size of 0.3 m x 0.3m. The inner diameter of well is 4.60 m and Outer diameter is 5.20 m.						
1	RCC quantity in Ring Beam	1	$(\pi/4) \times (5.20^2 - 4.60^2)$ sq. m.		0.3 m.	1.39 cu. m.
<b>Total Quantity of RCC in Ring Beam</b>						<b>1.39 cu. m.</b>

02 M  
(01 M for lift wise cal. And 01 M for its total)

02 M